

Application of cyclic accumulation models for undrained and partially drained general boundary value problems

A. M. Page Risueño

Yngres Dag 2014, May 15th 2014



Introduction

Cyclic loads in geotechnical engineering

- Offshore structures are subjected to static and cyclic loading due to:
Weight of the structure, wind, waves and current
- Under these load conditions,
the effect of **cyclic degradation** on the soil may be significant

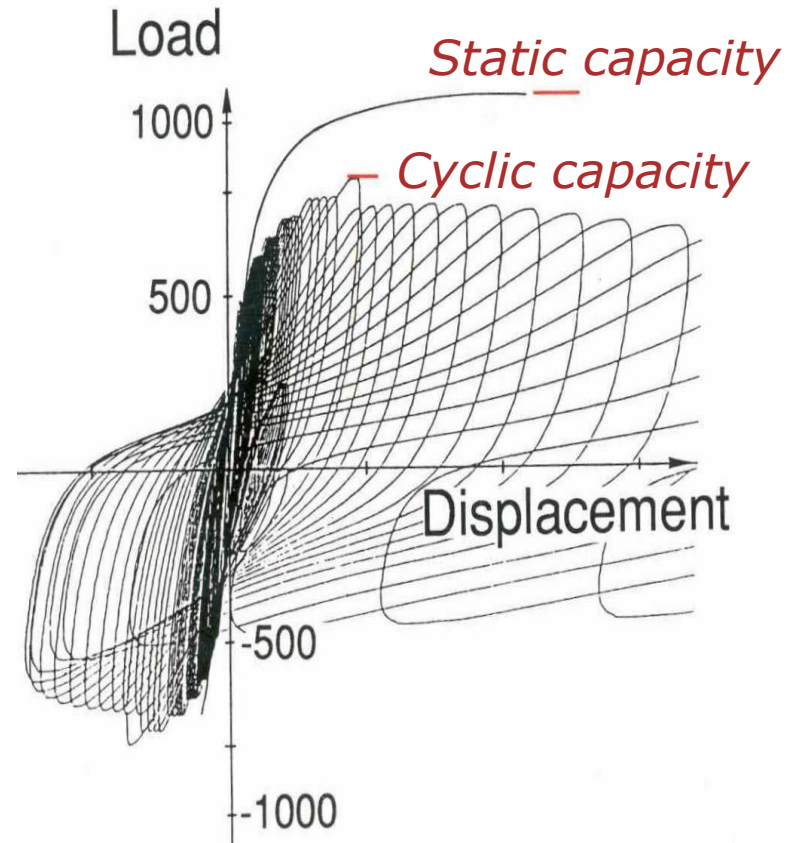


Introduction

Cyclic degradation of the soil

Effects of the **cyclic degradation** of the soil:

- Reduction in strength and stiffness
- Strain rate effects (clays)
- Accumulated pore pressure
- Accumulated deformations
- Damping

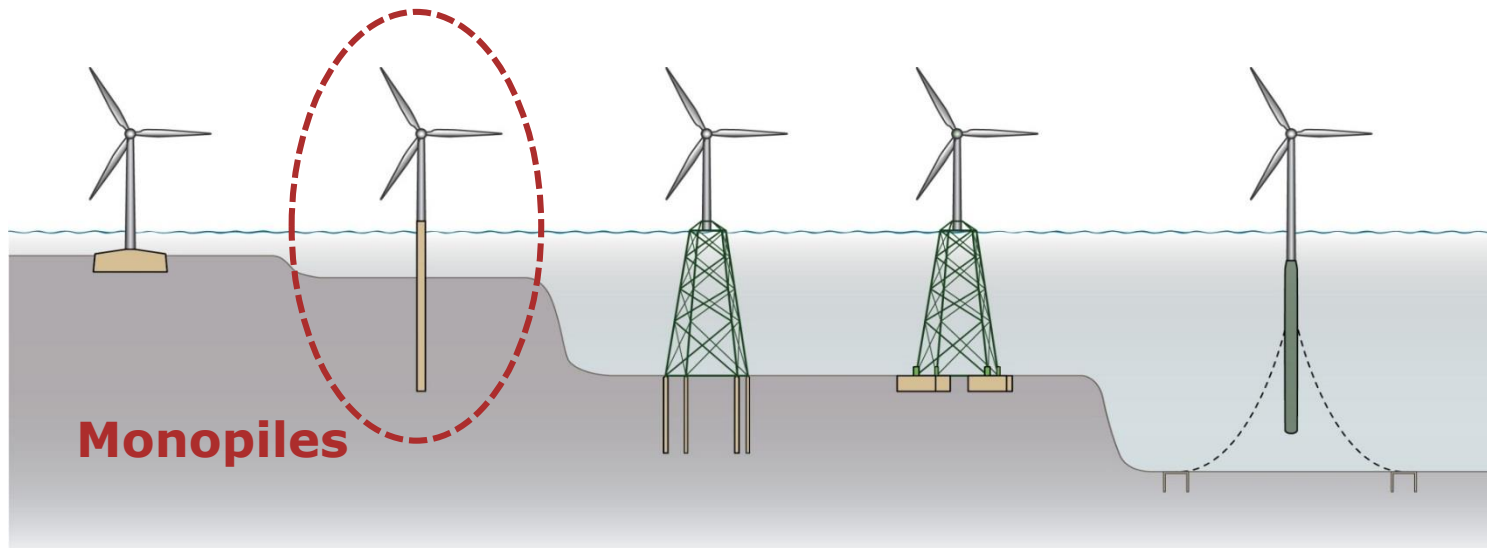


Results from a model test on a gravity platform on clay

Introduction

Cyclic degradation of the soil

The effect of cyclic loading has to be taken appropriately into account
in the design!



The cyclic degradation of the soil varies along the pile

How do we account for cyclic degradation?

Contour diagrams

The behaviour of the soil under cyclic loading is based on

Contour diagrams, which relate:

Cyclic shear stress, τ_{cy}

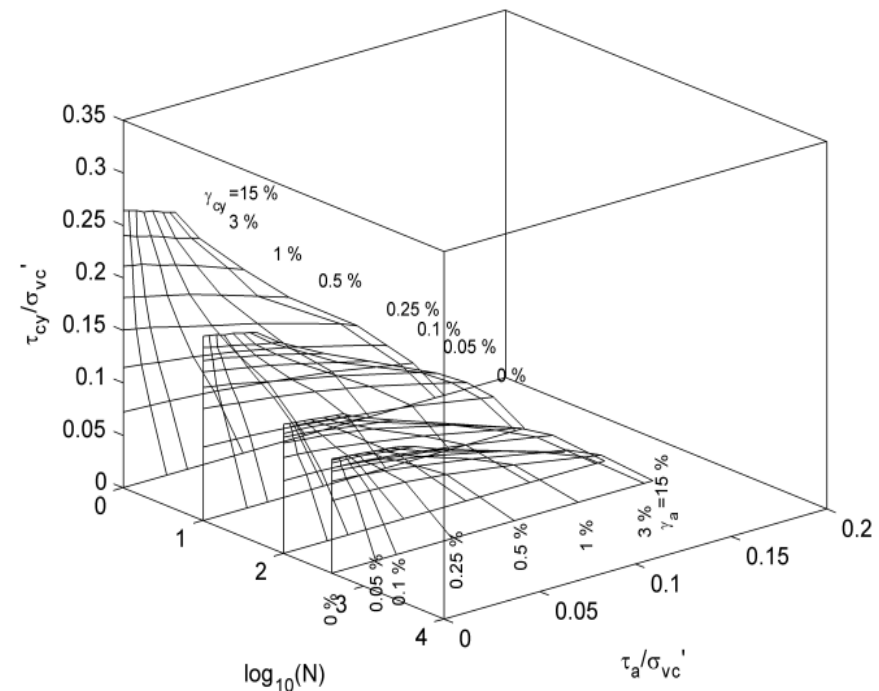
Cyclic shear strain, γ_{cy}

Average shear stress, τ_a

Average shear strain, γ_a

Number of cycles, N

Accumulated pore pressure, u_p

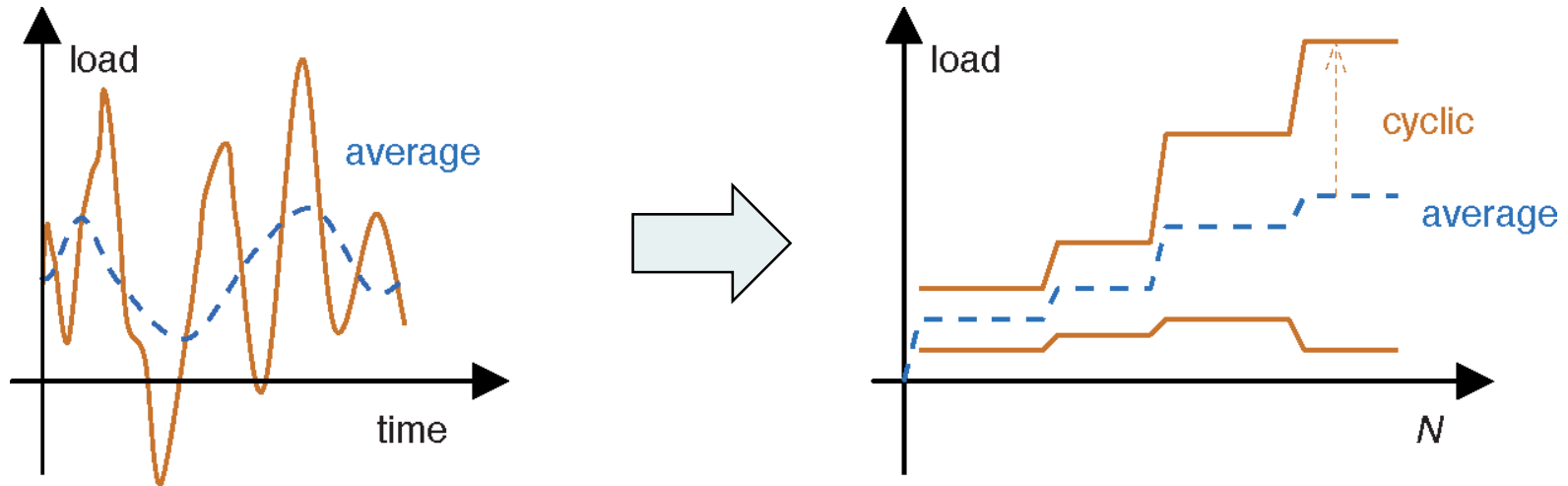


They are established from laboratory tests

How do we account for cyclic degradation?

Load application

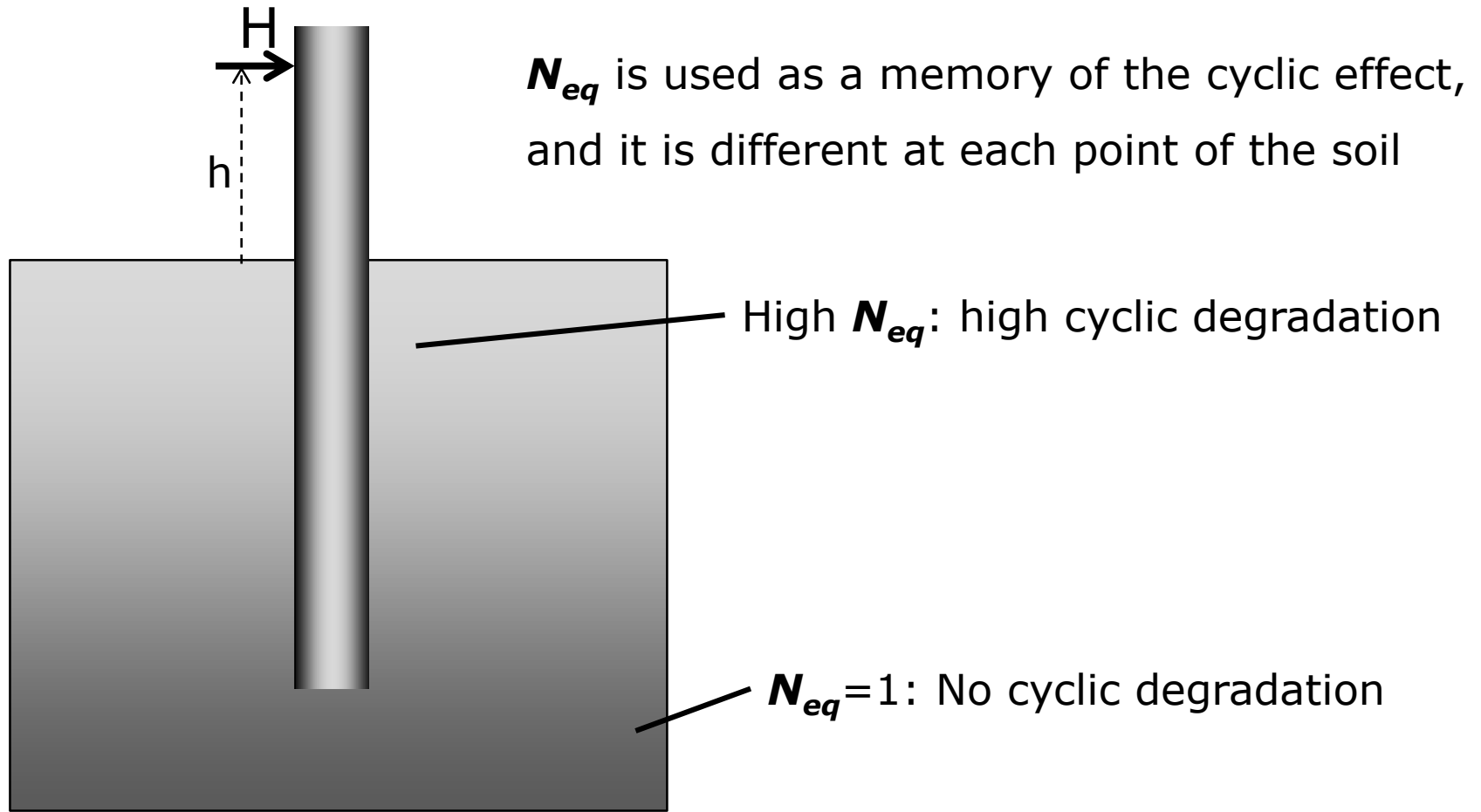
The real cyclic load history is rearranged in load parcels:



And the equivalent number of cycles, N_{eq} is calculated $\left\{ \begin{array}{l} \text{soil} \\ \text{geometry} \end{array} \right.$

How do we account for cyclic degradation?

Load application



Material models for cyclic response of soil

This procedure has been implemented as 2 user-defined models in PLAXIS:

- UDCAM: for undrained materials
- PDCAM: for partially drained materials

Material models for cyclic response of soil

UnDrained Cyclic Accumulation Model

Main features:

- Undrained behaviour under both average and cyclic loads (clays)
- Non-linear average and cyclic stress-strain relationships
- Cyclic degradation of stiffness and strength (N_{eq})
- Accumulated shear deformation
- Anisotropic behaviour (ADP)
- Based on input of laboratory results (interpolation and extrapolation between test results), instead of based on an elasto-plastic framework
- Implemented as a UDSM (DLL) in PLAXIS



Material models for cyclic response of soil

This procedure has been implemented as 2 user-defined models in PLAXIS:

- UDCAM: for undrained materials
- PDCAM: for partially drained materials

Material models for cyclic response of soil

Partially Drained Cyclic Accumulation Model

Used in coupled FE consolidation analyses (silts and sand)

Main features:

- Undrained behaviour during one single cycle
- Effective stress based (partly drained) relationship under the average loads
- Non-linear average and cyclic stress-strain relationships

Material models for cyclic response of soil

Partially Drained Cyclic Accumulation Model

Main features:

- Pore pressure accumulation due to cyclic loading
- Degradation of cyclic stiffness and cyclic strength (N_{eq})
- Accumulated shear and volume deformations
- Anisotropic behaviour (ADP)
- Based on input of laboratory results (interpolations and extrapolations)
- Implemented as a UDSM (DLL) in PLAXIS



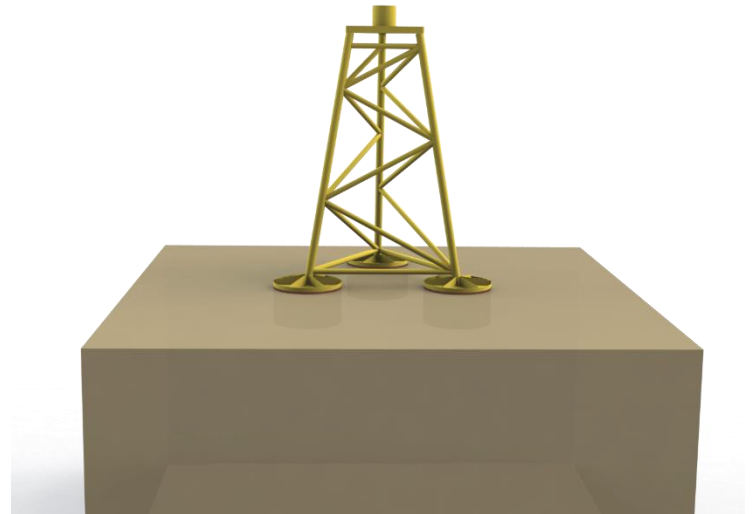
Applications

Comparison of the performance of UDCAM and PDCAM
with some simplified approaches

Case 1: Monopile foundation



Case 2: Bucket foundation

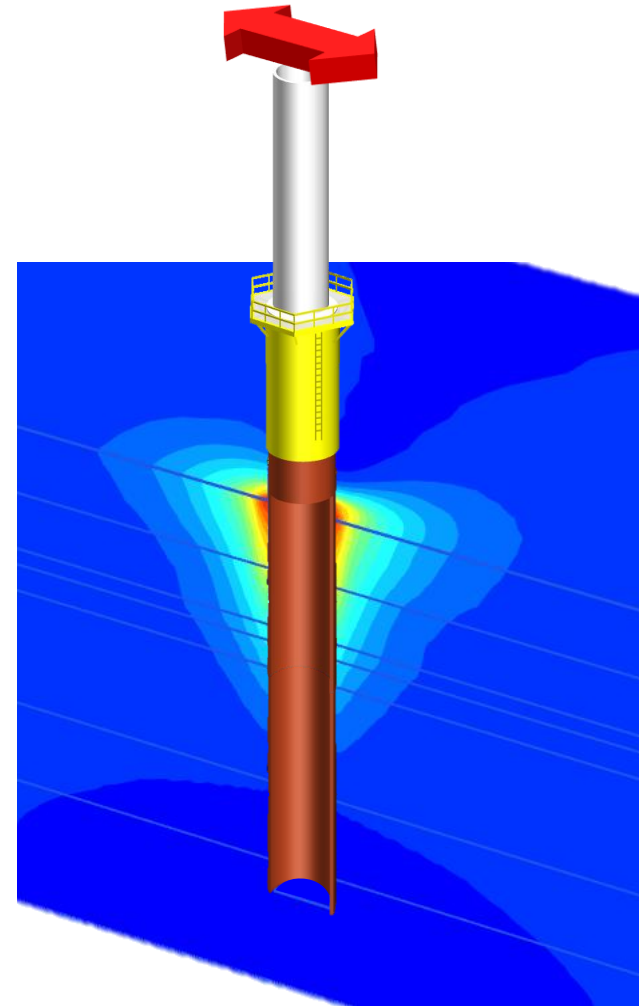


Applications

Monopile

Joint study between { GS E&C
NGI

Objective: see the effect of using more advanced models instead of p-y curves in the behaviour of a monopile



Horizontal displacements

Applications

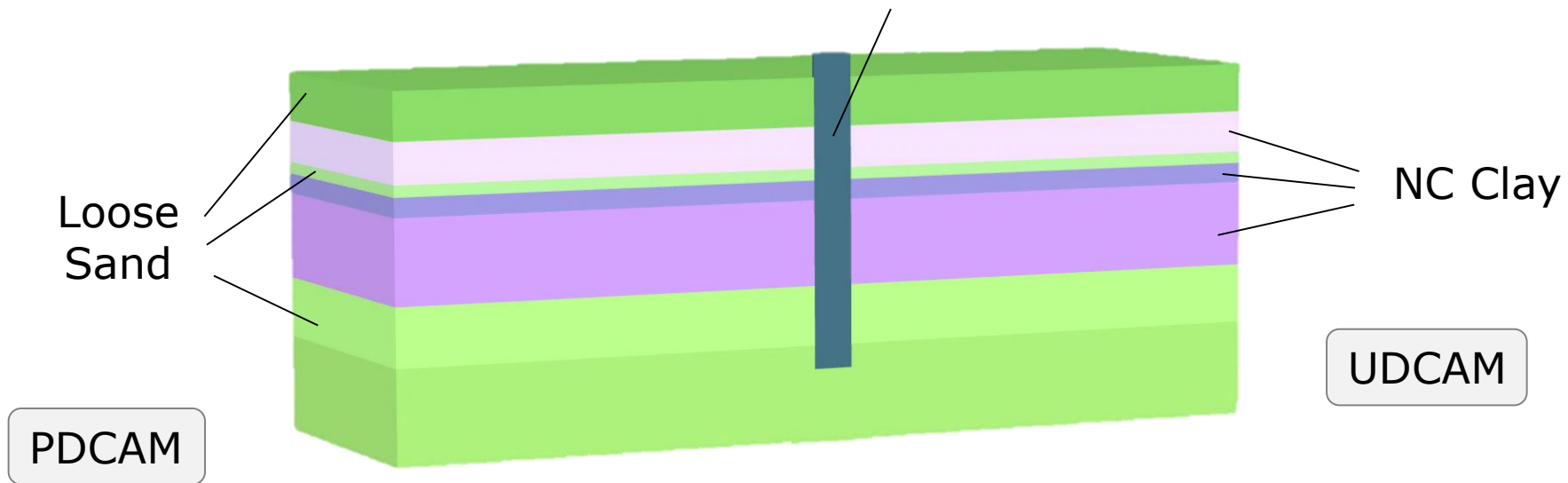
Monopile in a layered profile

Soil stratigraphy (Korean West Sea)

Software:

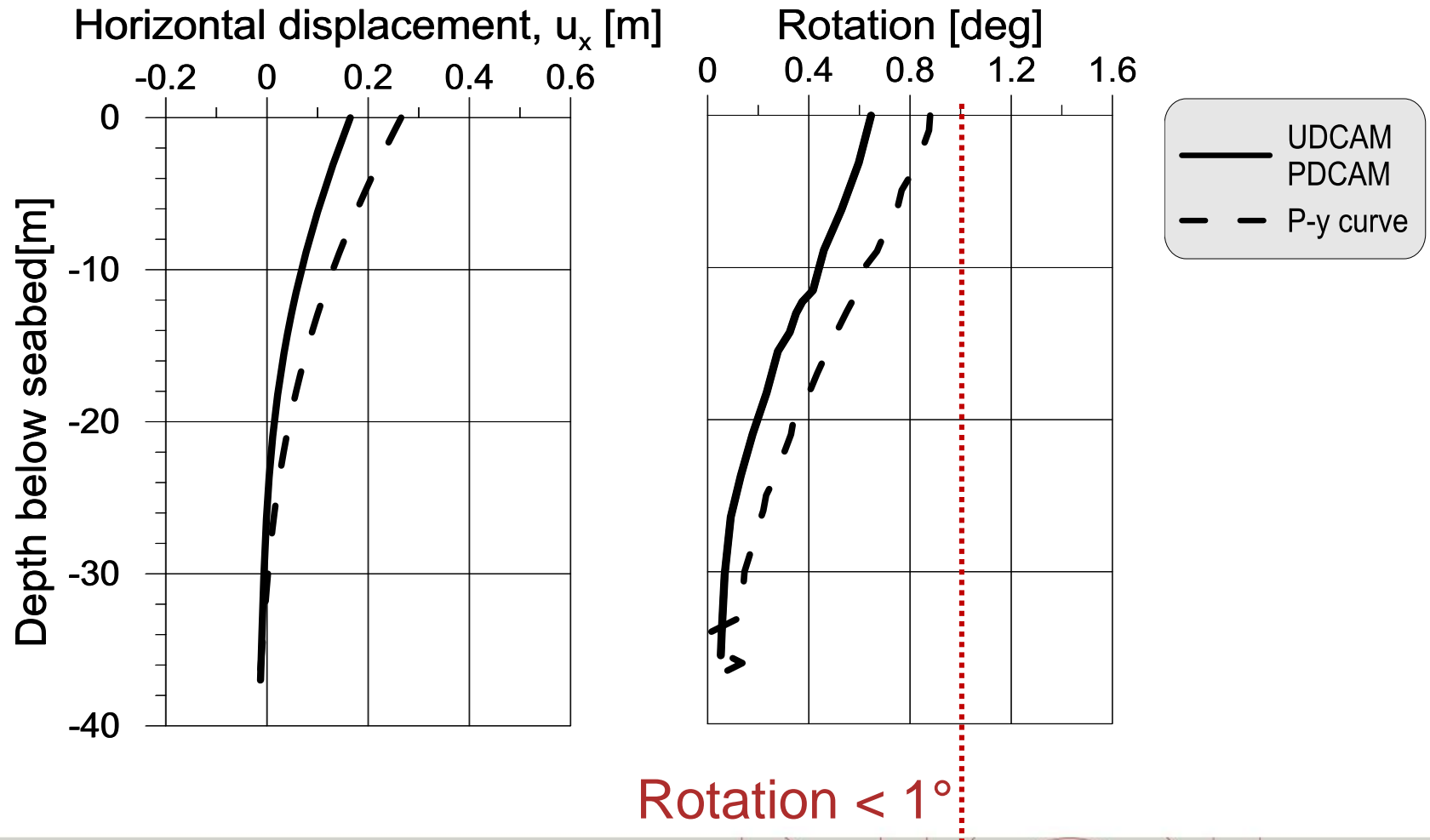
PLAXIS 3D Foundation

$d = 5.2 \text{ m}$
 $L = 37 \text{ m}$



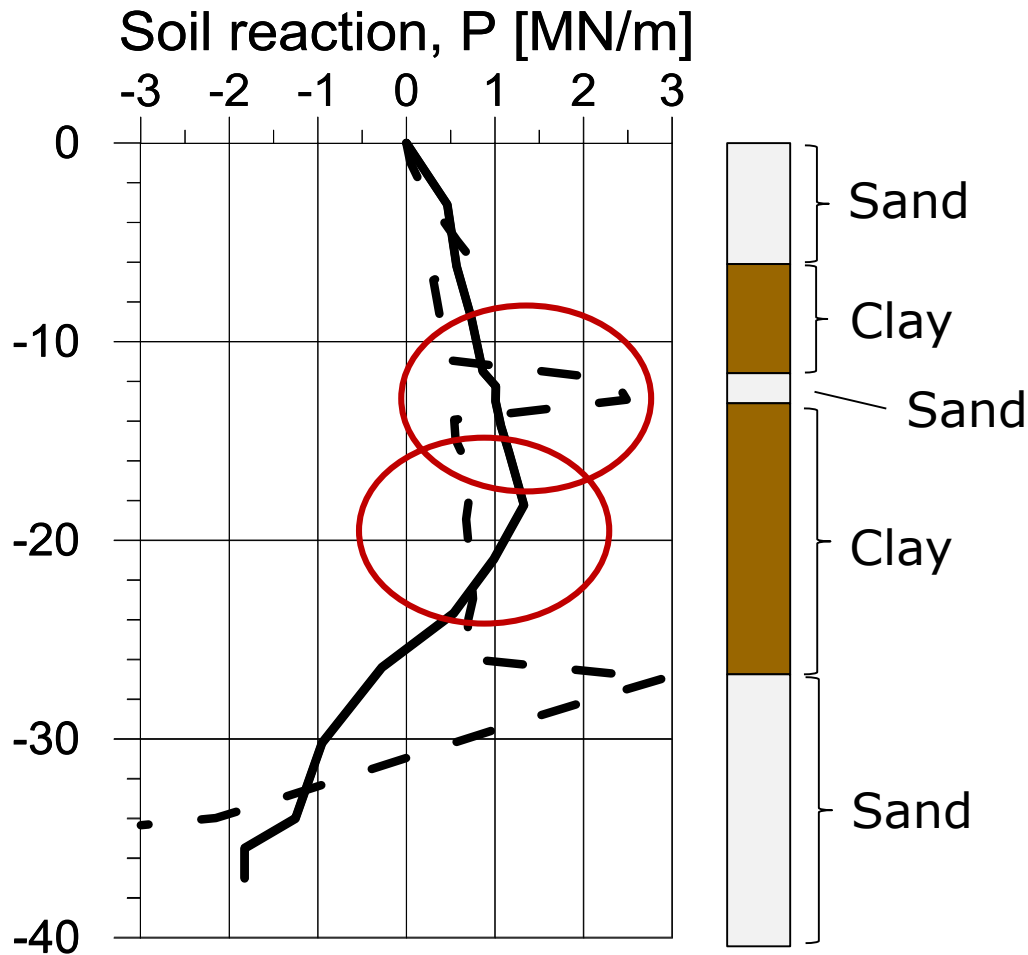
Applications

Monopile in a layered profile



Applications

Monopile in a layered profile



In clay:

The stiffness is higher in UDCAM than in the p-y curves

In sand:

The stiffness is lower for PDCAM than for the p-y curves

— UDCAM / PDCAM
- - P-y curve

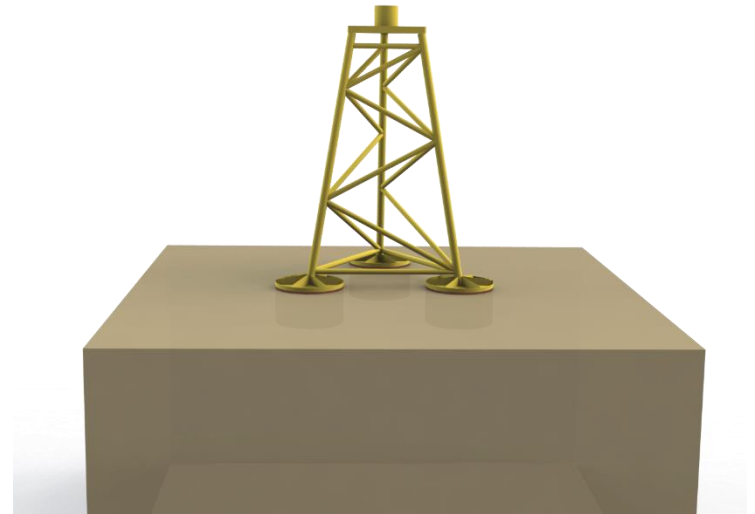
Applications

Comparison of the performance of UDCAM and PDCAM
with some simplified approaches

Case 1: Monopile foundation

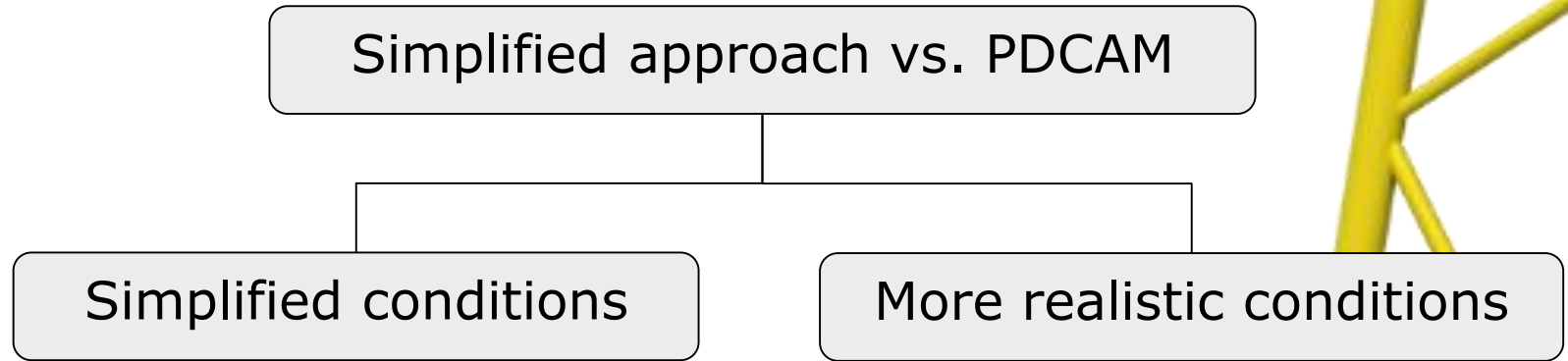


Case 2: Bucket foundation



Applications

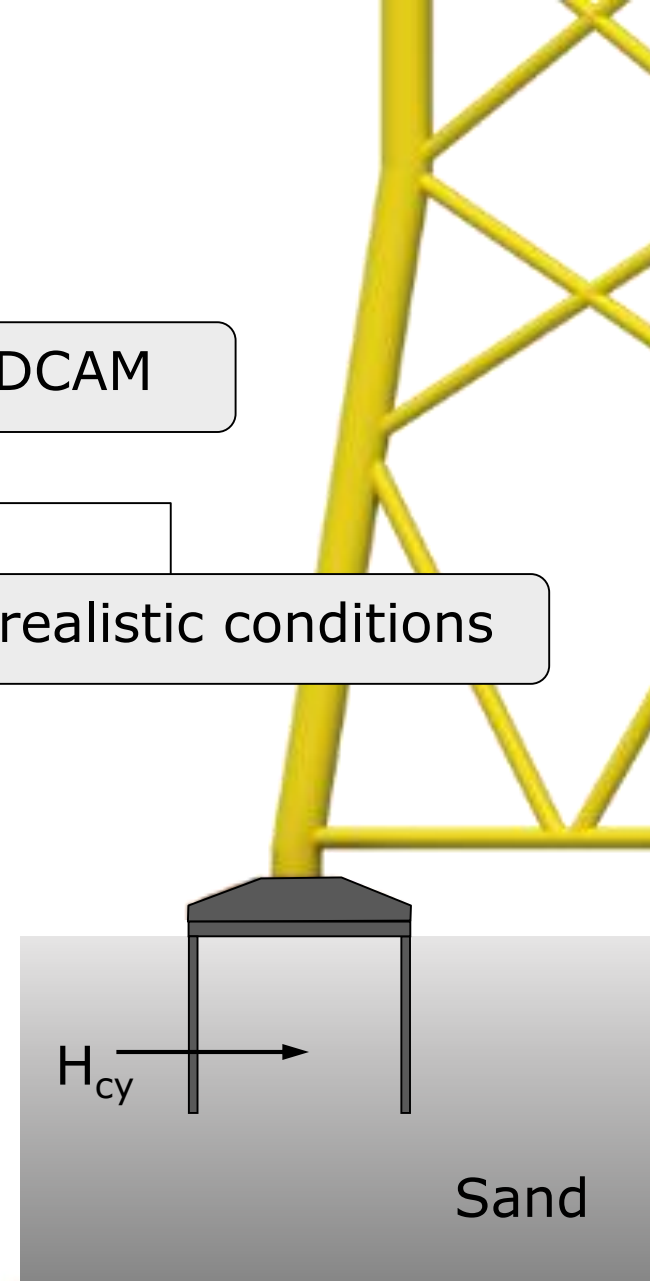
Bucket foundation



Compare:

- N_{eq}
- the accumulated pore pressure

For the same cyclic load history



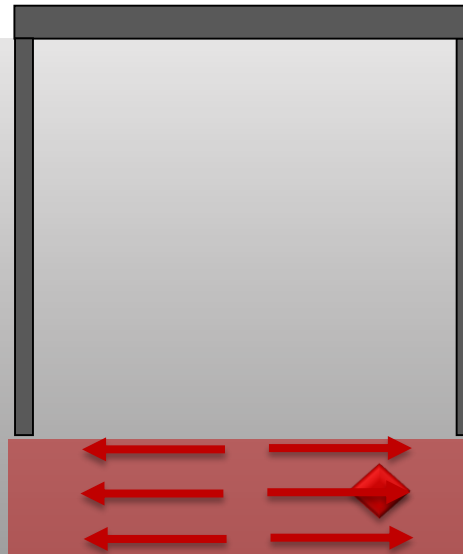
Applications

Bucket foundation

Simplified conditions

Assumptions:

Pore pressures
generated only
below the bucket



Linear-elastic soil

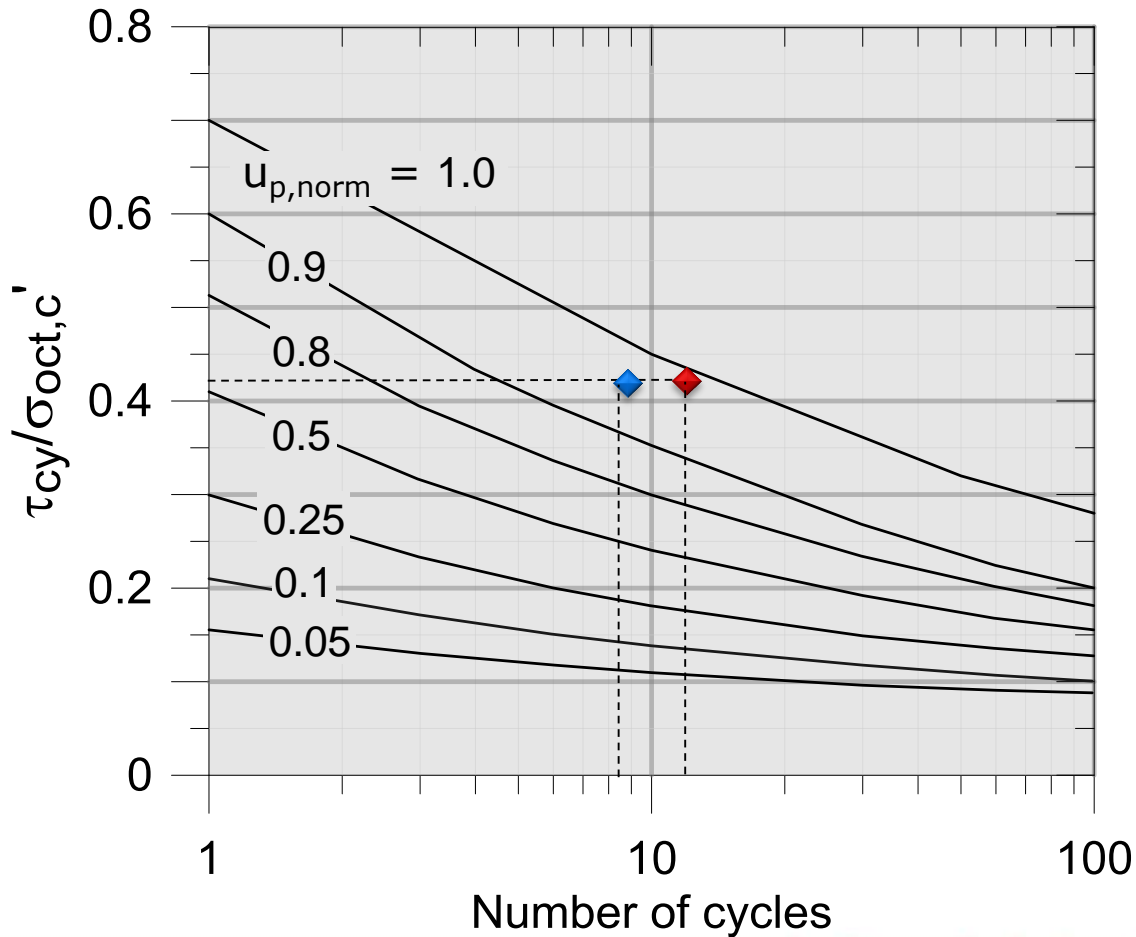
Only radial
disipation

The cyclic degradation is evaluated in one representative point

NGI

Applications

Bucket foundation



◆ PDCAM

◆ Simplified

Both procedures

(under similar
conditions)

give similar results

Applications

Bucket foundation

PDCAM: more realistic conditions

Assumptions:

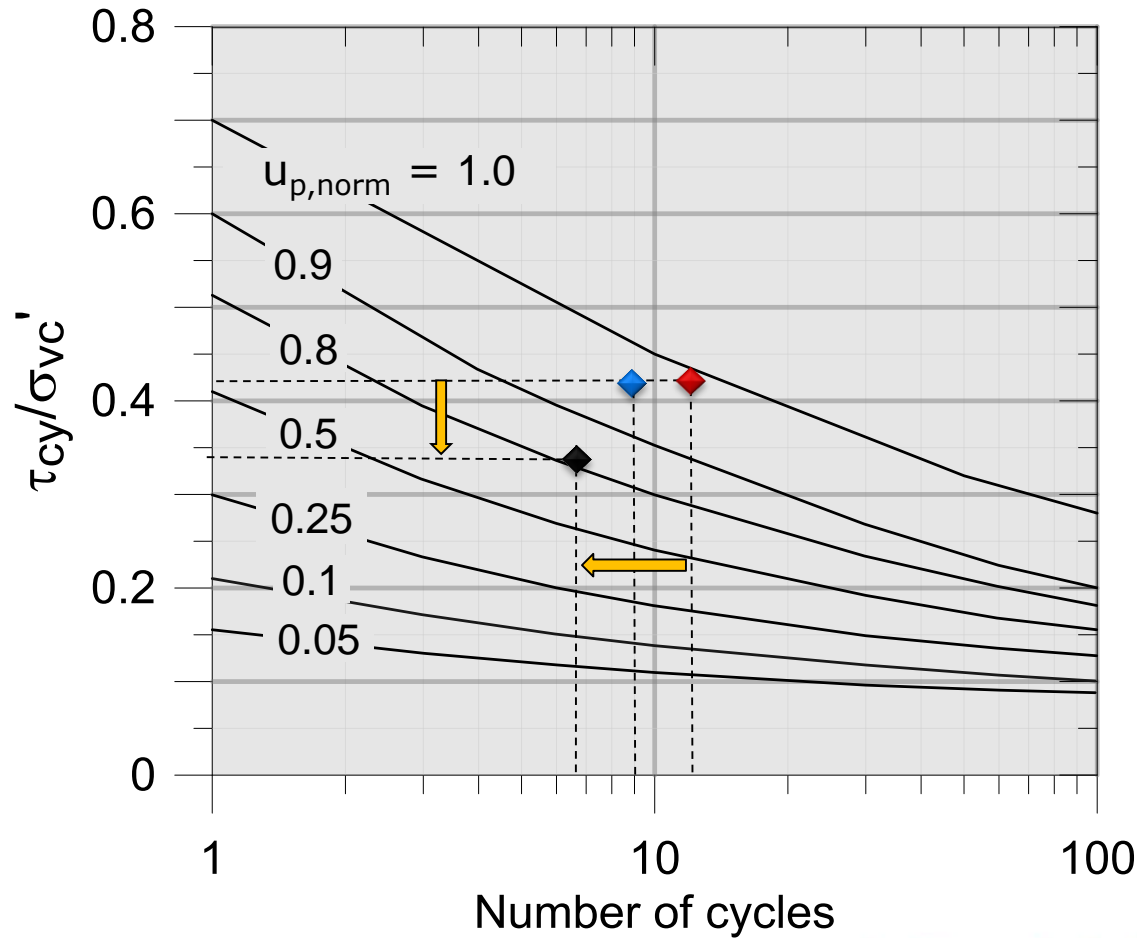
- soil with non-linear stress-strain relationship
- pore pressure generation and dissipation in any area and direction

We apply the same cyclic load history



Applications

Bucket foundation



◆ Simplified

◆ PDCAM
(simplified
conditions)

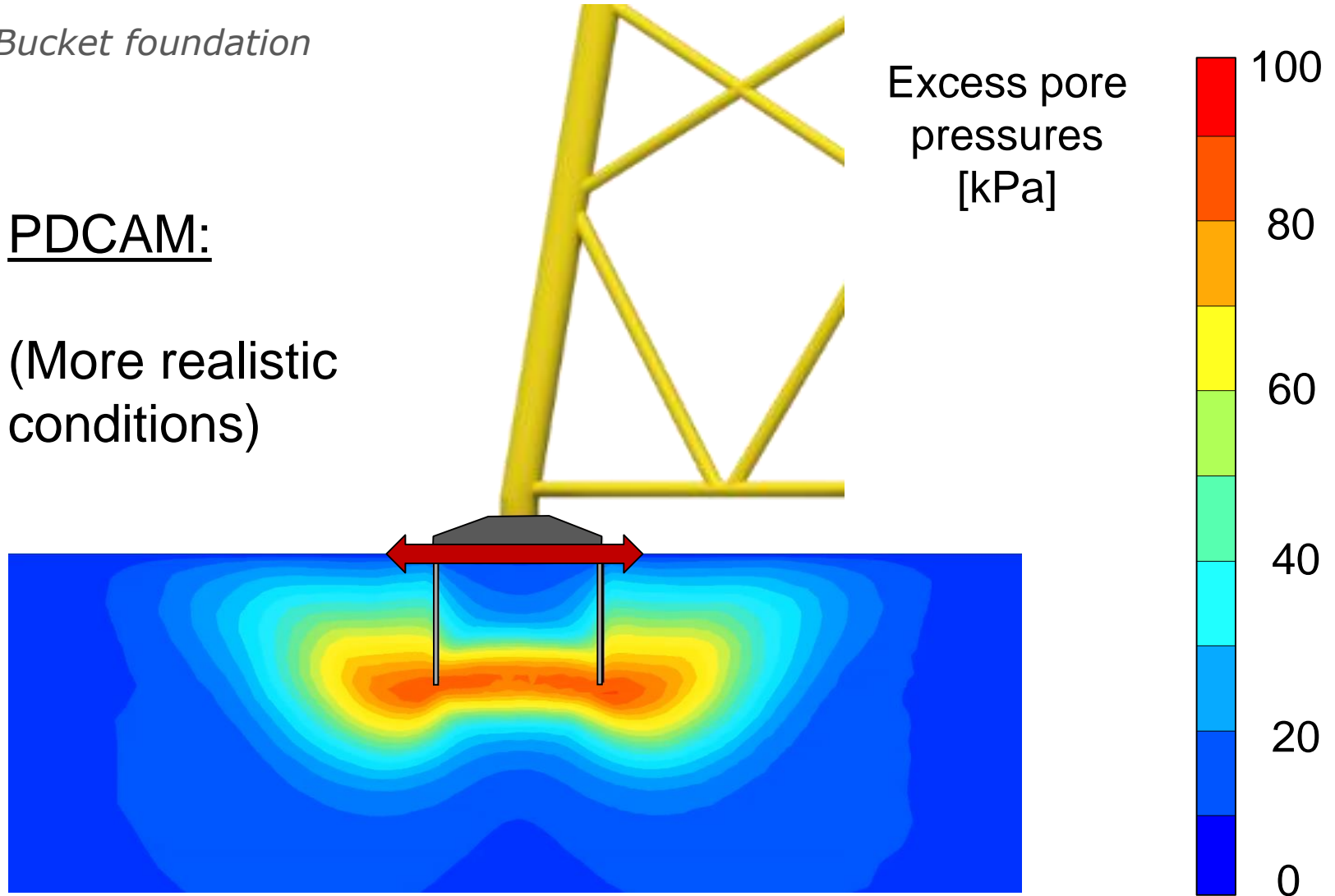
◆ PDCAM (more
realistic conditions)

Applications

Bucket foundation

PDCAM:

(More realistic conditions)



Conclusions

- 2 material models (UDCAM/PDCAM)
 - ↳ account for the cyclic degradation of the soil
 - ↳ implemented in a Finite Element program
- The models are suitable when
 - + the cyclic degradation of the soil varies along the structure
 - + stratigraphy with undrained and partially drained behavior

Conclusions

- Comparison p-y curves vs. UDCAM and PDCAM:
 - ↳ UDCAM and PDCAM can be both stiffer and softer than standard (API) p-y curves
- Comparison simplified approach vs. PDCAM:
 - + Both procedures, under similar conditions, give similar results
 - + By using more realistic assumptions, we can quantify the conservatism

Thank you for your attention!

